



### ADDENDUM 3

**ADDENDUM NO: 3**

**IMPROVEMENTS to PAVILION LAKE PIERS,  
DOCKS and GANGWAYS**

**DATE: 4/6/2020**

**BID NO: 01-2020**

**BID OPENING DATE: April 9<sup>th</sup>, 2020 @ 4:00PM.**

**NUMBER OF PAGES: 6**

This Addendum to the drawings, specifications and contract documents is issued to provide additional information and clarification to the original bid specifications and bid form and is hereby declared a part of the original drawings, specifications and contract documents. In case of conflict, this Addendum shall govern.

Bidders shall acknowledge receipt of this Addendum by inserting this Addendum in the attachment section of the Bid Form.

#### Description: RFI Questions and Answers

1. Is it possible at this point for you to confirm that the concrete fill in the truline wall is to be 6000 psi? I'm reading some conflicting info in the bid package.  
**MUE Response: yes, that is the case.**
2. Where is the New seawall being placed in relation to the existing wooden bulkhead?  
**MUE Response: at the same location.**
3. After driving the truline 800 sheetpile how much of the material is to be excavated prior to concrete fill?  
**MUE Response: All o fit.**
4. Plan sheet c-05 and c-06 show the sheetpile at approx 15' long. The manufacture is stating that it should be 20-22 feet for this project please clarify.  
**The cross sections on sheets C-05 and C-06 indicate the relationship of the seawall, boardwalk and fencing. Please refer to the manufacturer's recommendations for sheet pile lengths.**
5. The project is only accessible by a small narrow bridge. What is the weight limit for this bridge for purpose of bringing in a crane, trucks etc.  
**Refer to Question 3.1.1 of Addendum No. 2 dated March 31, 2020.**
6. Plans Page S-2.00, Detail 1 – Is the 2 ft. x 2 ft. crushed stone sump around the weep holes continuous for the total length of wall or are those individual sumps at each drain location? If individual at each drain please specify their length along the wall  
**MUE Response: this applies continuous along the complete length of the new seawall.**
7. Plans Page S-2.00, Detail 1 – What elevation or depth below the bottom of the concrete cap does the filter fabric extend to?  
**MUE Response: it extends 18" below the weep hole or 12" below the crushed stone.**

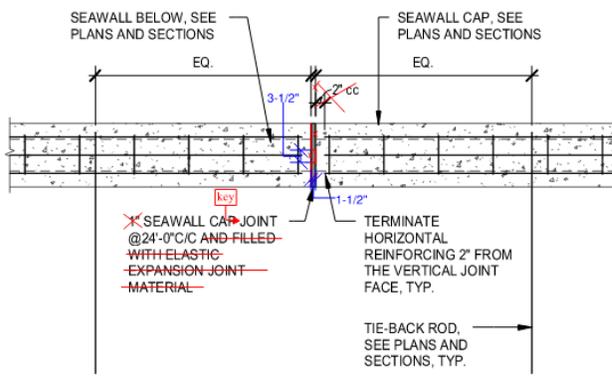


8. Plans Page S-2.00, Detail 1 – Does the filter fabric extend down the landside face of the sheet wall and then wrap around the crushed stone drain sump and then extend below the sump? Please clarify and provide dimensioning for the fabric.

**MUE Response:** Yes it does. The filter fabric is continuous along the complete length of the wall and extends from 18" below the weep hole to the bottom of the cap.

9. Plans Page S-0.00, Note 315000-3 - Please provide a detail of the keyed control joints that are required at a maximum spacing of 12 feet on center in concrete cap.

**MUE Response:** The keyed control joints shall be placed 24 ft (not 12 ft as mentioned in note 315000) on center and must be located centered between anchors. The keyed joint shall be a vertical keyed joint located at the center of the cap; The keyed joint shall be 3-1/2" wide and 1-1/2" deep. The cap longitudinal reinforcing must be terminated at each side of the keyed joint.



**B** TYPICAL SEAWALL CAP JOINT DETAIL  
S-1.00 1/2" = 1'-0"

10. Plans Page S-0.00, Note 315000-3 – “Keyed control joint spacing of 12 ft on center (midway between anchor rods)” will not work as anchors are spaced at 8 ft on center. Please clarify.

**MUE Response:** Please see our response to question 9 above.

11. Plans Page S-0.00, Note 315000-3 states 1-inch expansion joints are required at 48 ft. OC but Plans Page S-1.00, Detail B shows joints at 24 ft. OC. Which detail or note governs?

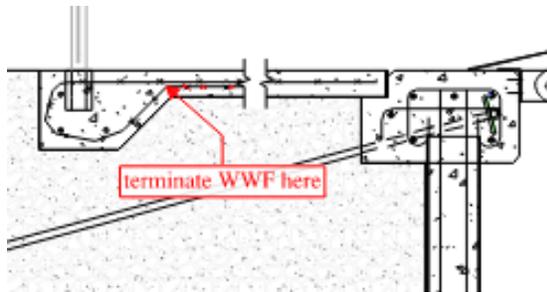
**MUE Response:** The cap key joints at 24 ft only. No additional expansion joints are required.

12. Plans Page S-1.00, Floor Schedule Mark 4" SOG states that sidewalk requires full penetration expansion joints at maximum 10 ft. OC but Foundation Floor Plan Note #8 states that sidewalks must have full depth expansion joints or 1/3 saw cut control joints at no more than 8 ft. OC. Please confirm that 1/3rd of depth crack control joints can be used at 8 ft. OC instead of having any full depth expansion joints.

**MUE Response:** the sidewalk requires a full penetration joint at 10ft on center. Sheet note #8 does not apply and will be removed from the CDs.

13. Plans Page S-2.00, Detail 4 – Appears that the WWF reinforcing is wrapping around 180 degrees through the thickened edge and the returning into the primary 4" sidewalk for some unknown distance. Please confirm our understanding is correct and specify the return length required.

**MUE Response:** That is correct. The bottom return of the WWF must only extend back to the 4" SOG section but does not have to be continued further; See snapshot below for additional clarification.



14. Plans Page S-2.00, Detail 4 – Would the engineer allow WWF reinforcing mats to be two discontinuous horizontal pieces and not wrap vertically at the thickened edge?

**MUE Response:** Yes, but the two mats must be spliced a minimum of 6”

15. Plans Page S-2.00, Detail 4 – Appears there are 3 each continuous longitudinal rebars in the thickened edge portion of the sidewalk but no callout for rebar sizes. Page S-1.00, Foundation Floor Plan Note # 11 states that there are 235 horizontal bars. What does this mean and please clarify qty. and bar size?

**MUE Response:** 235 is a typo. The thickened edge should be reinforced with 3 #4 rebars;

16. Plans Page S-2.00, Detail 4 shows a 12” x 12” thickened edge but Plans Page S-1.00, Foundation Floor Plan Note # 11 states 8” x 8”. Which detail or note governs?

**MUE Response:** it is a 12” wide x 8” deep thickened slab reinforced with 3#5 rebars

17. In general, there are many conflicting details and notes between S-0.00 Notes, S-1.00 Plan, and S-2.00 Sections and Details. If conflicts are not specifically asked or answered in the bidding Q&A can you provide which drawings take precedence over the others in the event of discrepancies.

**MUE Response:** please see responses to questions above and let us know if there are is any other conflicting information that needs to be addressed.

18. Due the volume of unanswered questions nearly a week away from bid we respectfully requested a bid extension to earliest April 9, 2020 to give bidders and their subs and suppliers time to digest potential clarifications.

**Refer to Bid Addendum No. 2, dated March 31, 2020. The bid date has been extended to April 9, 2020 at 4:00 PM.**

19. Plans Page C-08, Floating Dock & Gangway Note 1 – Floating dock suppliers are not familiar with “UV Rating 80” specification. Could you please further clarify what this means for them.

**Each flotation shall be composed of hand welded High Density Polyethylene (HDPE) sheet plastic, using non-rotationally molded floats. Each section is manufactured using individual sheets for sides, ends, bottoms, and tops, with a 100% guaranteed universal wall thickness of .150 inches (min.), and completely encapsulated expanded polystyrene (EPS) foam. The lid or top surface has a 2-1/2” lip around the entire float so that each section can bolt to the bottom the aluminum framing. HDPE plastic will be black in color and have a levant non-skid texture. It will incorporate an ultraviolet inhibitor of UV-8 or better. All plastic material meets requirements of ASTM D4976 – PE 235 & FDA 21CFR 177.1520.**



20. Plans Page C-08, Floating Dock & Gangway Note 8 – Gangway suppliers are asking if Engineer can allow a lesser live load rating on gangway as 100 psf for 6 ft. x 50 ft. gangway is 30,000 lbs. Could this requirement be closer to a standard 50 or 60 psf rating?

**AccuDock aluminum gangways and ramps are built with 6005-T5 marine grade aluminum to customer's desired length and width. All AccuDock gangways and ramps are built to withstand a 50 pounds per square foot live load rating. Greater rating can be achieved if specified by the buyer.**

21. Plans Page S-2.00, Detail 2 states that 14" PCP piles shall be installed minimum 20 ft. into slightly clayey to silty fine sand layer. Based on the soil borings provided on Page G-03 and bathymetric data provided on Page C-05 the mudline at locations of the PCP piles is approximately EL -10 (and at depth 30 ft. in boring profiles). This means pile tips go to EL -30 (boring depth 50 ft). The only soil layers between EL -10 and EL -30 are "silty sand with shell and limestone fragments" and the "hard limestone" layer (50+ blows) a few feet above boring EL -30. Please specify if Contractor will need to predrill and penetrate into "hard limestone" to get to pile tips to EL -30 or can they terminate driving once refusal is met via conventional impact driving around EL -27 or -28?

**MUE Response: Drilling thru "hard limestone" might be required if solid limestone layers are encountered at the pile locations. A pile embedment of 17 ft into silty sandy soils might be acceptable depending on the soil composition at the pile location but would have to be evaluated at the time of the pile installation.**

These answers were missing from Addendum No. 3.

22. Plans Page C-08, Floating Dock & Gangway Note 1 – Floating dock suppliers are not familiar with "UV Rating 80" specification. Could you please further clarify what this means for them.

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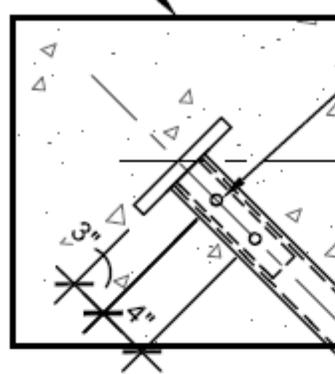
24. We have another question regarding the helicals that came from a subcontractor.

1. See attached detail. This would remove the all-thread from the scope of work. Could this be used in lieu of the currently called out pile components?

## 2. THE BALANCE OF THIS PAGE IS INTENTIONALLY BLANK

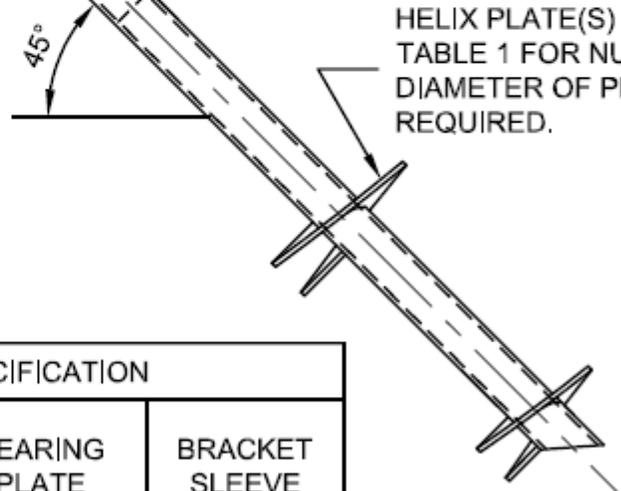


REF. STRUCTURAL DWGS FOR BEAM DIMN & REINF.



HELICAL TIEBACK BRACKET (REF. TABLE 1)

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HELIX PLATE(S) TABLE 1 FOR NU DIAMETER OF P REQUIRED.

TABLE 1 - HELICAL TIEBACK AND BRACKET SPECIFICATION

PILE MARK	PILE SIZE	ALLOW. CAPACITY (KIPS)	HELIX CONFIG.	MIN. TORQUE (FT-LBS)	BEARING PLATE	BRACKET SLEEVE
C5 <sup>2</sup>	3 1/2" Ø	52	8"-10"-12"-14"	13,000	PL 1"x 9"x 0'-9"	2 7/8" Ø x 10"
C6 <sup>3</sup>	2 7/8" Ø	26	8"-10"-12"	7,000	PL 5/8"x 8"x 0'-8"	3 1/2" Ø x 10"
D5	2 1/4" x 2 1/4"	102	8"-10"-12"-14"	23,000	PL 1"x 9"x 0'-9"	NOTE 1

NOTES:

- 1) DOUBLE WALLED SQUARE TUBE (HSS 5/16"x 3" x3" & HSS 5/16"x 3.5"x 3.5") 10 INCHES LONG.
- 2) CONNECT BRACKET TO PILE WITH (2) 3/4"Ø SAE GRADE 8 THRU BOLTS.
- 3) CONNECT BRACKET TO PILE WITH (2) 1"Ø SAE GRADE 8 THRU BOLTS.



See Addendum No. 1 – refer to RFI numbers: 2, 19, 21, 22, 23 and 24. An alternate anchor detail such as the one suggested below is in general an acceptable alternate option but it most likely will require a larger cap and it will have to be modified to a 15 degree angle as per the current design.

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**CITY OF BELLE GLADE**

**BIDDER**

\_\_\_\_\_  
**NEIL APPEL, C.P.M.**  
**PURCHASING MANAGER**

\_\_\_\_\_  
**Signature**

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**Printed, Title**

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**COMPANY**